

## **APPENDIX II**

EUSTIS ENGINEERING COMPANY, INC.

**LOG OF BORING AND TEST RESULTS**  
 HALTER MARINE SERVICES, INC.  
 CRANE PAVING AND MISCELLANEOUS IMPROVEMENTS  
 LOCKPORT, LOUISIANA

(Sheet 1 of 2)



Ground Elev.:

Datum:

Gr. Water Depth: See Text

Job No.: 15405

Date Drilled: 5/20/98

Boring: 5

Refer to "Legends &amp; Notes"

Scale In Feet	PP	SPT	S P L R	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits				Other Tests			
										Dry	Wet	Type	$\phi$	C	LL	PL	PI					
0	45	17		X	Compact gray clayey silt w/shell fragments	ML	1	0-0.5	9													
					Very stiff tan & brown silty clay	CL	2	1-2														
					Medium stiff gray silty clay	CL	3	3-4														
				T	Medium compact gray sandy silt	ML	4	5-6	25	94	118	UC	-	435								
							5	8-9	34	86	115	UC	-	515								
							6	11-12	29	94	121											
							7	14-15														
							8	18-19	30	89	116							29	24	5		
							9	23-24														
							10	28-29	33	84	112	UC	-	265								
							11	31-32														
				T	Soft gray silty clay	CL	12	34-35	29	90	116	OB	--	670								
					Loose gray clayey silt	ML	13	38-39														
					Medium dense gray clayey sand	SC	14	43-44	30	85	110	OB	-	525								
					Medium stiff gray sandy clay	CL	15	48-49	20													
					Loose gray silty sand w/clay layers	SM																

Comments:

EUSTIS ENGINEERING COMPANY, INC.

**LOG OF BORING AND TEST RESULTS**  
 HALTER MARINE SERVICES, INC.  
 CRANE PAVING AND MISCELLANEOUS IMPROVEMENTS  
 LOCKPORT, LOUISIANA

(Sheet 2 of 2)



Ground Elev.:

Datum:

Gr. Water Depth: See Text

Job No.: 15405

Date Drilled: 5/20/98

Boring: 5

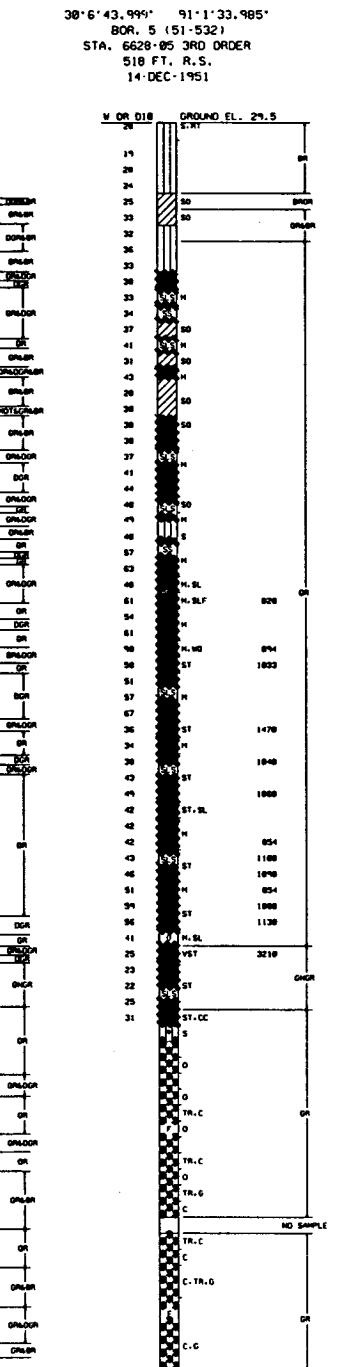
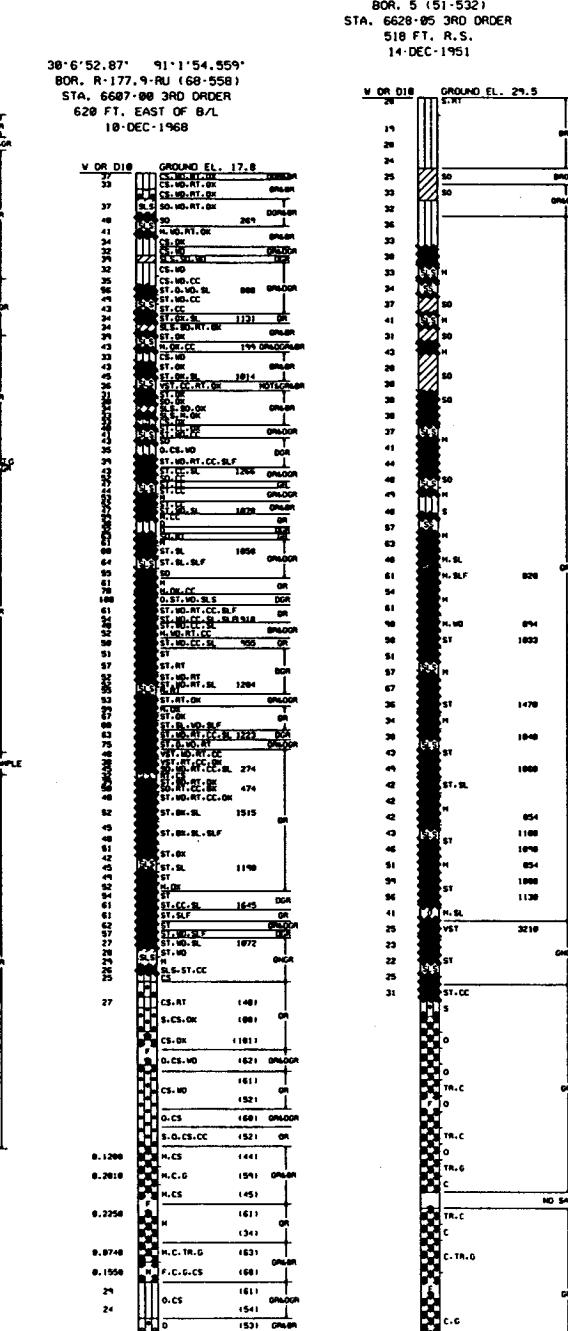
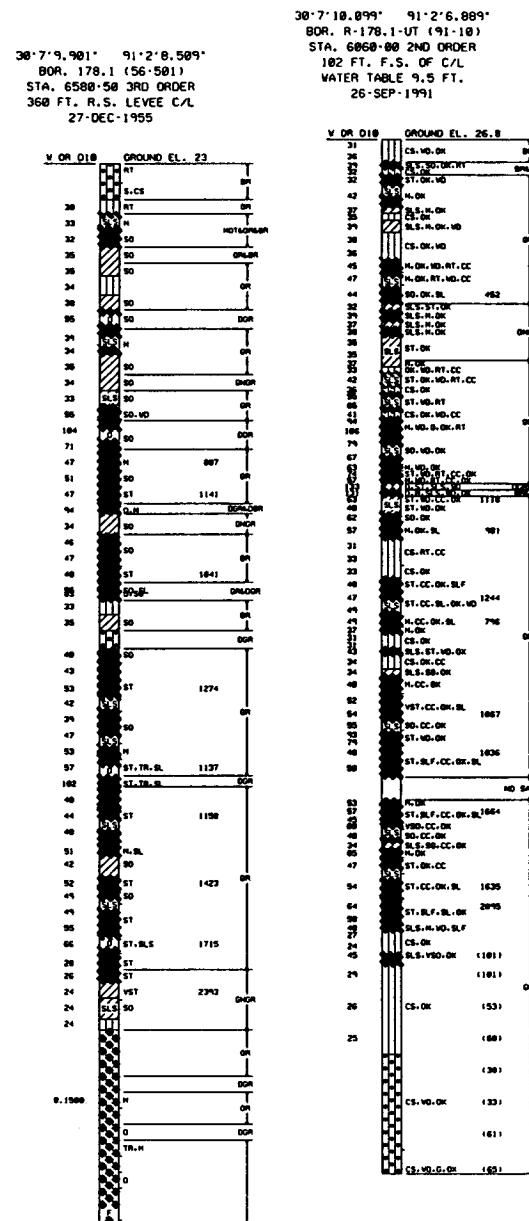
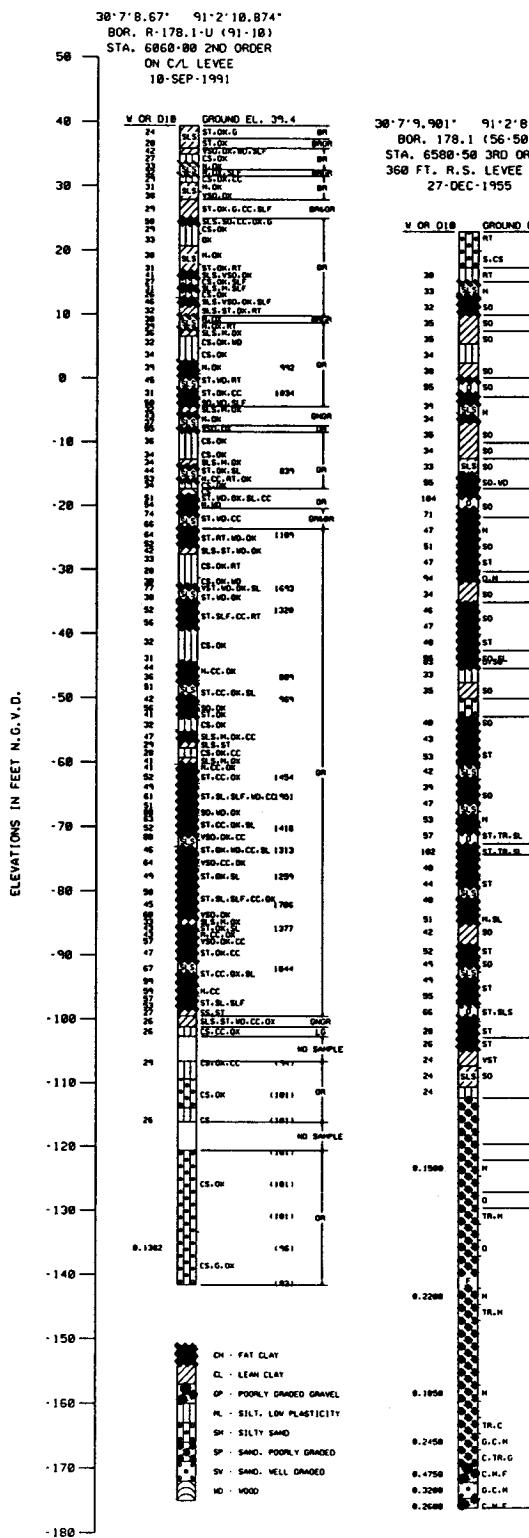
Refer to "Legends &amp; Notes"

Scale In Feet	PP	SPT	S P L R	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits				Other Tests			
										Dry	Wet	Type	$\phi$	C	LL	PL	PI					
50	0.25	8	T	X	Loose gray silty sand w/clay layers	SM																
					Medium stiff gray clay w/shell fragments & sand lenses	CH				16	53-54	51	69	105	UC	-	790					
										17	58-59											
					Medium stiff gray silty clay	CL				18	63-64	34	82	110	UC	-	535					
					Medium stiff gray clay	CH				19	68-69											
										20	73-74	54	67	103	UC	--	720					

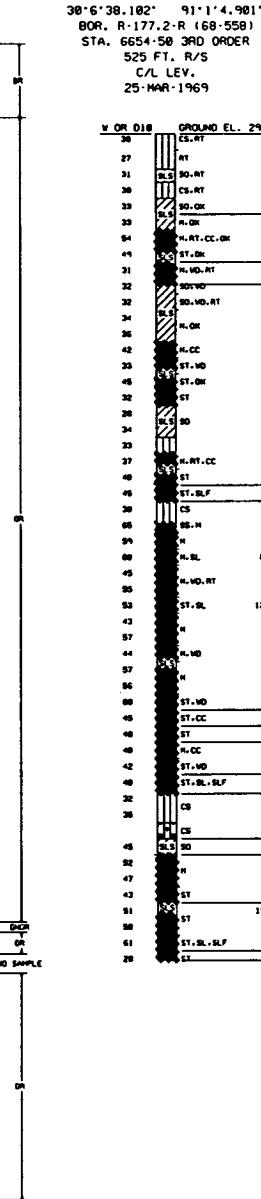
Area 1

(1 of 1)

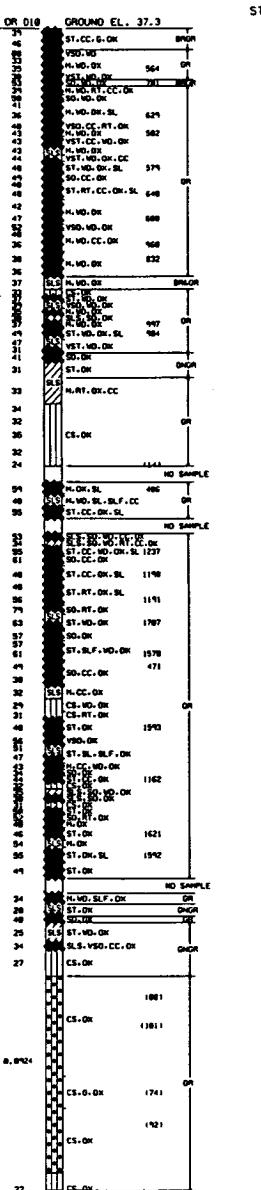
1' = 30'



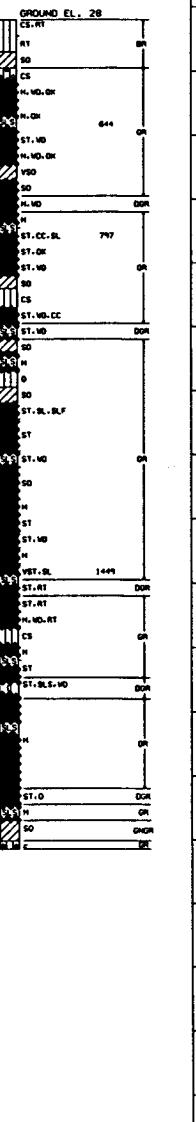
30°6'39.388" 91°1'31.182"  
BOR. R-177.5-U (91-10)  
STA. 6110-00 2ND ORDER  
C/L LEVEE  
WATER TABLE 14.8 FT.  
28-AUG-1991



30°6'31.111" 91°0'37.793"  
BOR. R-176.7-U (91-10)  
STA. 6169-00 2ND ORDER  
C/L OF LEVEE  
WATER TABLE 12.8 FT.  
14-AUG-1991



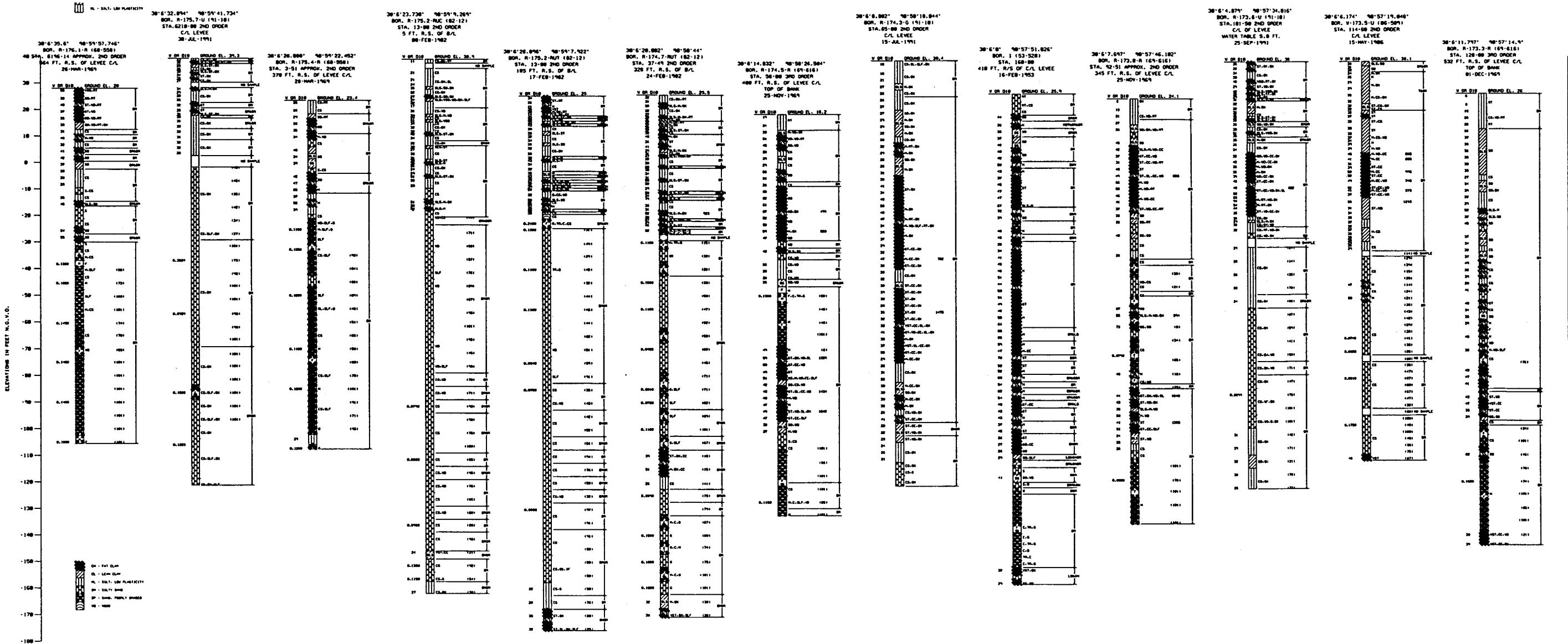
30°6'34.848" 91°0'38.182"  
BOR. R-176.6-R (68-558)  
STA. 6686-58 3RD ORDER  
672 FT R.S.  
21-MAR-1969



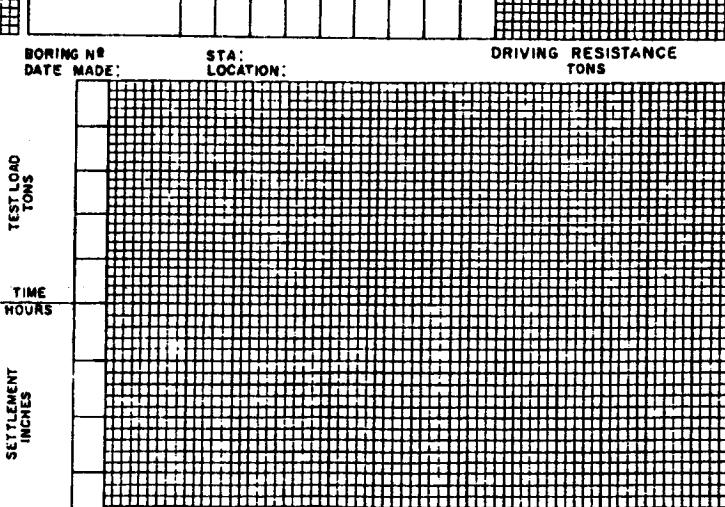
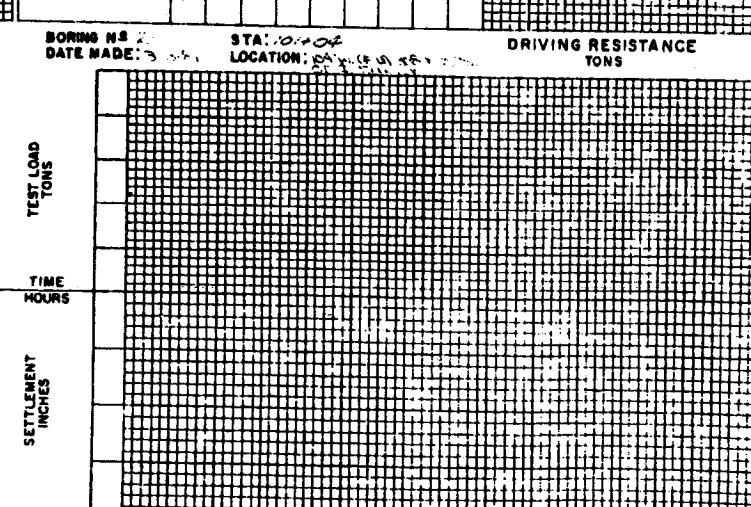
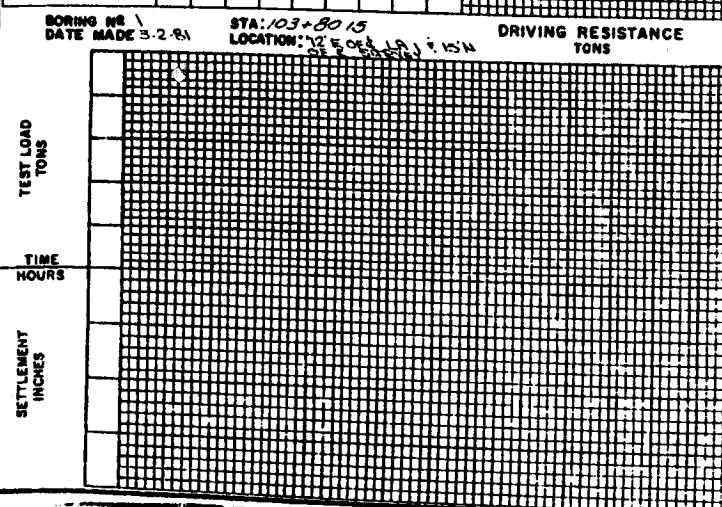
1' = 30'

## Area 3 (1 of 1)

$$l'' = 40'$$



SOIL TYPE & COLOR	40	6	TYPE BREAK	%	MOISTURE CONTENT	LIQUID LIMIT	PLASTIC INDEX	SALINITY NUMBER	TEST FILE NO.	
									TYPE OF HAMMER	TEST NUMBER
BR. S. CL. WTR. RE.	-	-	C50	21	40	22	(5)	15		
BR. S. CL. WTR. RE.	96	45	059	52	39	11	(5)	10		
BR. S. CL. WTR. RE.	96	45	059	52	39	11	(5)	5		
BR. S. SW. BR. CL. SA	5.00 5.0	60	060	29	28	2	(5)	0		
GR. SA. CL. WTR. RE.	0.88	MS	060	30	35	15	(5)	-10		
CL. SA.									-15	
GR. SA. CL. S. WTR.		-	059	-	N. P.	(5)				
GR. S. CL. WTR. RE.	2.71	45	058	33	43	19	(5)	-20		
GR. S. CL. S.	-	-	058	33	42	19	(5)	-25		
GR. S. CL. WTR. RE.	0.86	45	058	30	42	19	(5)	-30		
GR. CL. S. CL. WTR. RE.	0.86	60	060	30	29	3	(5)	-35		
GR. S. CL.	1.10	MS	068	35	45	20	(5)	-40		
GR. SA. CL. SA.	5.00 5.0	60	062	30	30	+	(5)	-45		
GR. S. CL. WTR. RE.	0.52	MS	068	28	46	25	(5)	-50		
GR. CL.	0.97	MS	062	41	104	61	(5)	-55		
WTR. SH. & OGS @ 75' 100'								-60		
	1.66	MS	062	53	-	-		-65		
WTR. SH. & OGS 120' - 125'								-70		
								-75		
								-80		
								-85		
								-90		
								-95		
								-100		
								-105		
								-110		



AS BUILT

NOTES	F.A.P.	STATE PROJECT	PARISH	SW
	BRO-005(035)	803-29-02	ASCENSION	114

- |                         |                                                                                                                                                                                                                                                    |
|-------------------------|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| <b>MOISTURE CONTENT</b> | <ul style="list-style-type: none"> <li>• Moisture Content of the material in its natural state expressed as a percentage of the dry weight of the material</li> </ul>                                                                              |
| <b>(3)</b>              | <ul style="list-style-type: none"> <li>• Location and Identification of the sample</li> </ul>                                                                                                                                                      |
| <b>NO PENETRATION</b>   | <ul style="list-style-type: none"> <li>• Unable to drive split spoon sampler initial 6 inches</li> </ul>                                                                                                                                           |
| <b>N + 10</b>           | <ul style="list-style-type: none"> <li>• Number of stabs of 140 lbs., hammer dropped 30 in., required to drive 2 in. O.D. Split Spoon sampler 1 ft., after first having been driven 6 in., unless amount of penetration shown otherwise</li> </ul> |
| <b>NO CULL</b>          | <ul style="list-style-type: none"> <li>• No preliminary 6" driving prior to securing drive data</li> </ul>                                                                                                                                         |
| <b>C</b>                | <ul style="list-style-type: none"> <li>• Compressive strength from unconsolidated undrained triaxial shear test (Tons per sq. ft.)</li> </ul>                                                                                                      |
| <b>QU</b>               | <ul style="list-style-type: none"> <li>• Compressive strength from consolidated undrained shear test (tons per sq. ft.)</li> </ul>                                                                                                                 |
| <b>f</b>                | <ul style="list-style-type: none"> <li>• Angle of internal friction</li> </ul>                                                                                                                                                                     |
| <b>y</b>                | <ul style="list-style-type: none"> <li>• Wet weight of in-place material (Tons per cu. ft.)</li> </ul>                                                                                                                                             |
| <b>s</b>                | <ul style="list-style-type: none"> <li>• Strength determined by consolidated drained triaxial shear test (Tons per sq. ft.)</li> </ul>                                                                                                             |
| <b>#</b>                | <ul style="list-style-type: none"> <li>• Strength determined by penetrometer</li> </ul>                                                                                                                                                            |

CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES				
SOIL	DESIGNATION	N <sup>o</sup> OF BLOWS "N"	"E <sub>u</sub> " UNCONFINED COMPRESSIVE STRENGTH TONS PER SQ FT	
SAND	RELATIVE DENSITY	VERY LOOSE LOOSE MEDIUM DENSE VERY DENSE	LESS THAN 4 4-10 10-30 30-50 OVER 50	
SILT				
CLAY	CONSISTENCY	VERY SOFT SOFT MEDIUM STIFF VERY STIFF HARD	LESS THAN 2 2-4 4-8 8-15 15-30 OVER 30	LESS THAN 0.25 0.25 - 0.50 0.50 - 1.00 1 - 2 2 - 4 OVER 4

**PROJECT ENGINEER**  
**CORE BORINGS AND TEST PILES.**

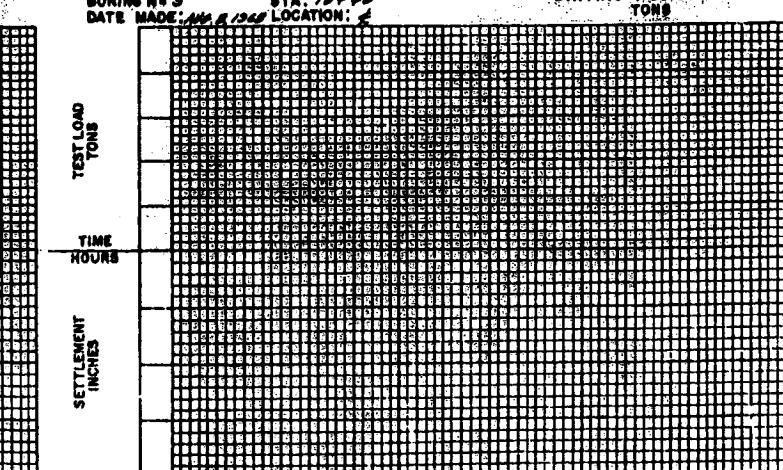
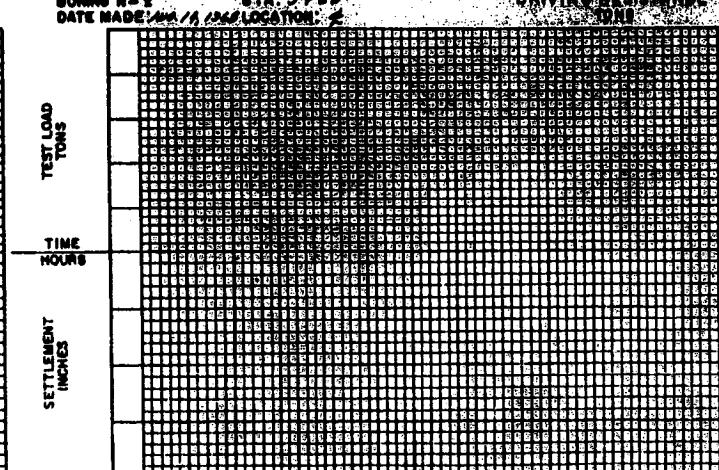
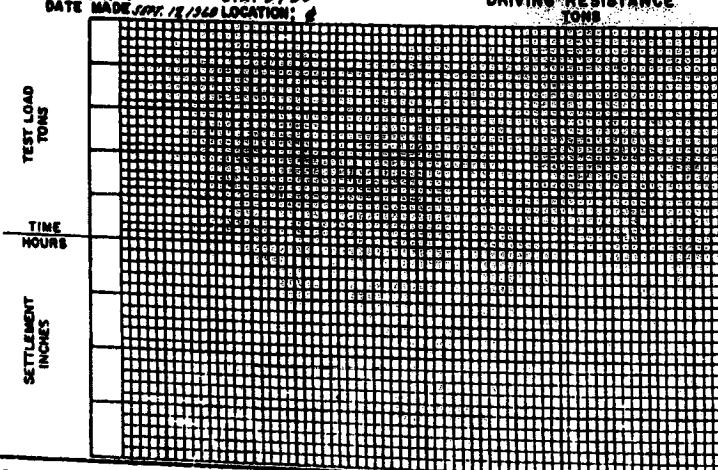
# BAYOU LAFOURCHE BRIDGE AND APPROACHES

ROUT

**STATE OF LOUISIANA**  
**DEPARTMENT OF TRANSPORTATION & DEVELOPMENT**

F. B. NO. 12-1421		DATED MAR. 13, 1961	STATE OF LOUISIANA DEPARTMENT OF TRANSPORTATION & DEVELOPMENT		
			SURVEYED	CHECKED	DETAILED
		BY	CHECKED	CHECKED	TRACED
DATE	DESCRIPTION				
	REVISIONS				
	MATERIAL REQUESTED	RECORDED	TRANSMITTED	DESIGN SECTION	

CLASSIFICATION				TEST FILE NO.			
SOIL TYPE & COLOR	GR.	CONSISTENCY	MOISTURE CONTENT	Liquid Limit	Plasticity Index	Sample No.	
SILTY CLAY LOAM			9	27	6	10	20
SILTY CLAY LOAM			10	44	4	11	21
SILTY CLAY LOAM	STAB	13	31	11			
SL. SILTY TO SANDY CLAY LOAM	STAB	16	23	6			
SL. SILTY CLAY - DR. LOAM	MUD	29	33	15			
SL. SILTY TO SANDY CLAY LOAM	0.75	MUD	30	36	9		
SL. SILTY WITH CLAY LOAM	0.65	MUD	29	37	6		
SL. SILT WITH LOAM		LOAM	30	41	11	12	22
SL. SILT WITH CLAY LOAM		MUD	29	41	11	13	23
SL. SILT WITH CLAY LOAM		MUD	28	41	11	14	24
SL. SILTY CLAY LOAM		MUD	29	41	11	15	25
SL. SILTY CLAY LOAM		MUD	30	41	11	16	26
SL. SILTY CLAY LOAM		MUD	29	41	11	17	27
SL. SILTY CLAY LOAM		MUD	29	41	11	18	28
SL. SILTY CLAY LOAM		MUD	29	41	11	19	29
SL. SILTY CLAY LOAM		MUD	29	41	11	20	30
SL. SILTY CLAY LOAM		MUD	29	41	11	21	31
SL. SILTY CLAY LOAM		MUD	29	41	11	22	32
SL. SILTY CLAY LOAM		MUD	29	41	11	23	33
SL. SILTY CLAY LOAM		MUD	29	41	11	24	34
SL. SILTY CLAY LOAM		MUD	29	41	11	25	35
SL. SILTY CLAY LOAM		MUD	29	41	11	26	36
SL. SILTY CLAY LOAM		MUD	29	41	11	27	37
SL. SILTY CLAY LOAM		MUD	29	41	11	28	38
SL. SILTY CLAY LOAM		MUD	29	41	11	29	39
SL. SILTY CLAY LOAM		MUD	29	41	11	30	40
SL. SILTY CLAY LOAM		MUD	29	41	11	31	41
SL. SILTY CLAY LOAM		MUD	29	41	11	32	42
SL. SILTY CLAY LOAM		MUD	29	41	11	33	43
SL. SILTY CLAY LOAM		MUD	29	41	11	34	44
SL. SILTY CLAY LOAM		MUD	29	41	11	35	45
SL. SILTY CLAY LOAM		MUD	29	41	11	36	46
SL. SILTY CLAY LOAM		MUD	29	41	11	37	47
SL. SILTY CLAY LOAM		MUD	29	41	11	38	48
SL. SILTY CLAY LOAM		MUD	29	41	11	39	49
SL. SILTY CLAY LOAM		MUD	29	41	11	40	50
SAND			26	41	10	41	51
SAND			26	41	10	42	52
SAND			26	41	10	43	53
SAND			26	41	10	44	54
SAND			26	41	10	45	55
SAND			26	41	10	46	56
SAND			26	41	10	47	57
SAND			26	41	10	48	58
SAND			26	41	10	49	59
SAND			26	41	10	50	60
SAND			26	41	10	51	61
SAND			26	41	10	52	62
SAND			26	41	10	53	63
SAND			26	41	10	54	64
SAND			26	41	10	55	65
SAND			26	41	10	56	66
SAND			26	41	10	57	67
SAND			26	41	10	58	68
SAND			26	41	10	59	69
SAND			26	41	10	60	70
SAND			26	41	10	61	71
SAND			26	41	10	62	72
SAND			26	41	10	63	73
SAND			26	41	10	64	74
SAND			26	41	10	65	75
SAND			26	41	10	66	76
SAND			26	41	10	67	77
SAND			26	41	10	68	78
SAND			26	41	10	69	79
SAND			26	41	10	70	80
SAND			26	41	10	71	81
SAND			26	41	10	72	82
SAND			26	41	10	73	83
SAND			26	41	10	74	84
SAND			26	41	10	75	85
SAND			26	41	10	76	86
SAND			26	41	10	77	87
SAND			26	41	10	78	88
SAND			26	41	10	79	89
SAND			26	41	10	80	90
SAND			26	41	10	81	91
SAND			26	41	10	82	92
SAND			26	41	10	83	93
SAND			26	41	10	84	94
SAND			26	41	10	85	95
SAND			26	41	10	86	96
SAND			26	41	10	87	97
SAND			26	41	10	88	98
SAND			26	41	10	89	99
SAND			26	41	10	90	100



NOTES:	R.A.R.	STATE PROJECT	P.
150	D-500 (D)	ESB-30-01	ASS

**ABBREVIATIONS:**

N.P.	NON-PLASTIC
BL.	BLUE
BK.	BLACK
BR.	BROWN
GR.	GRAY
WH.	WHITE
YE.	YELLOW
DK.	DARK
LT.	LIGHT
MED.	MEDIUM
PENCIL	PENETRATION
S/B	UNABLE TO TEST BECAUSE OF SLICKENRIDES

**MOISTURE CONTENT =** MOISTURE CONTENT OF THE MATERIAL  
IN ITS NATURAL STATE EXPRESSED  
AS A PERCENTAGE OF THE DRY WEIGHT  
OF THE MATERIAL.

**NO PENETRATION** : UNABLE TO DRIVE SPLIT SPOON SAMPLER INITIAL 6 INCHES.

N-10

NUMBER OF BLOWS OF 100 LB.  
HAMMER DROPPED 30 IN. REQUIRED  
TO DRIVE 2 IN. SPLIT SPOON SAMPLER  
1 FT. AFTER FIRST HAVING BEEN DRIVEN  
6 IN. UNLESS AMOUNT OF PENETRATION  
SHOWN OTHERWISE.

**NO CULL** • NO PRELIMINARY 6" DRIVING PRIOR TO SECURING DRIVE DATA.

CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES				
SOIL	DESIGNATION	NO OF BLOWS "N"	"N"	INCONFINED COMPRESSIVE STRENGTH TONS PER SQ.
SAND OR SILT	VERY LOOSE		LESS THAN 4	
	LOOSE		4-10	
	MEDIUM		10-30	
	DENSE		30-50	
CLAY	VERY DENSE		OVER 50	
	SOFT		LESS THAN 2	LESS THAN
	SOFT		2-4	0.25-0.50
	MEDIUM		4-6	0.50-1.00
	STIFF		6-15	> 1.00
	STIFF		15-30	> 2
	HARD		OVER 30	OVER 4

CORE BORINGS AND TEST PILES. BRIDGE NO. I

BAYOU LAFOURCHE BRIDGE  
INCOURTVILLE - SUNSHINE BRIDGE HWY  
La 70

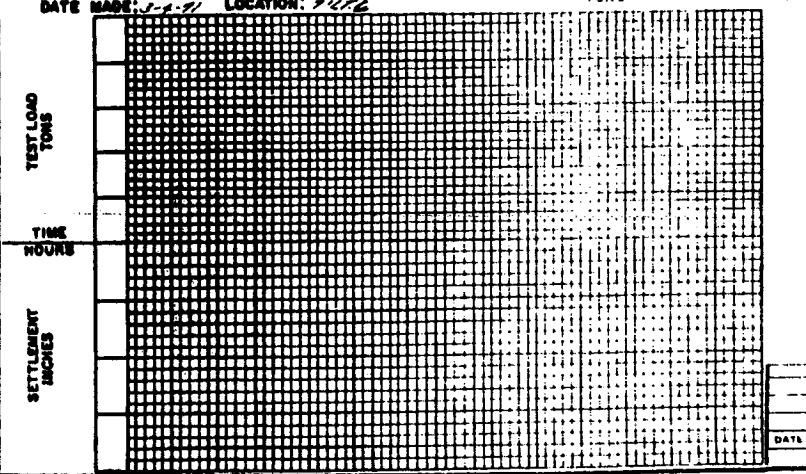
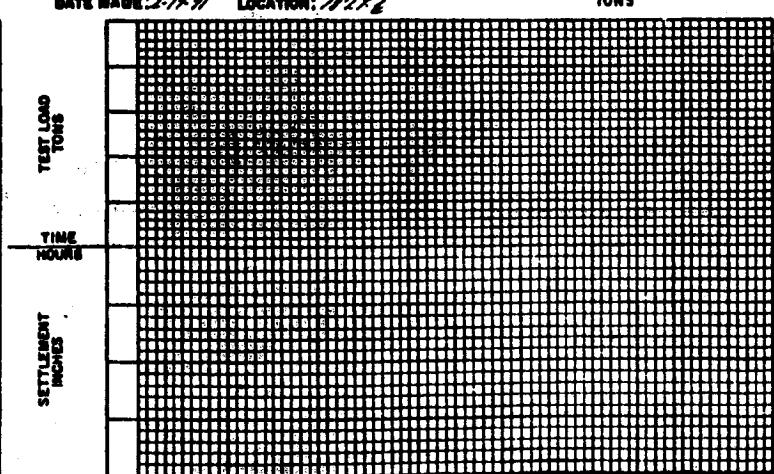
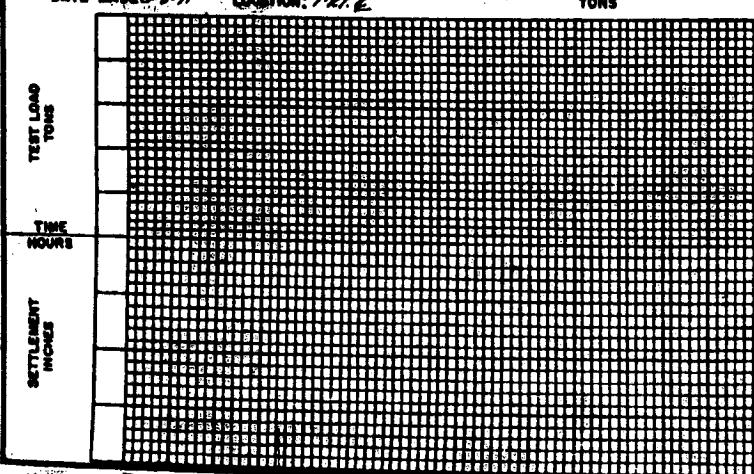
215 " 268

**STATE OF LOUISIANA  
DEPARTMENT OF HIGHWAYS**

B.E.G. 6	DIALED	TRACED <i>MAP</i>
JAN. 1	-	CHEK'D <i>ST</i>
TESTING SECTION - BRIDGE DESIGN SECTION		

SHEET 1 OF 7

SOIL TYPE & COLOR	QU	QT	TYPE BREAK	MOISTURE CONTENT	LIQUID LIMIT	PLASTICITY INDEX	SAMPLE NUMBER	ELEVATION	TEST FILE NO.	
									STATION	FT. L.
GR. CLY. SILT W/1% OGD-570	0.66	10.0	100 27	32 16	50	16	50	-60		
GR. CLY. SOY. SILT	0.10	10.0	100 29	34	P	16	50	-10		
GR. SI. CL. W/1% OGD-570	0.00	10.0	100 31	31	7	16	50	-2		
GR. CLY. SOY. SILT	1.13	10.0	100 31	34	P	16	50	-1		
GR. SI. CL. W/1% OGD-570	0.80	10.0	100 32	33	11	16	50	-11		
GR. CLY. SI. S.A.	1.49	10.0	100 29	34	P	16	50	-20		
GR. CLY. SOY. SILT	0.86	10.0	100 30	33	13	16	50	-20		
GR. SI. CL.	0.71	10.0	100 30	34	26	16	50	-20		
GR. CL.	0.55	10.0	100 35	30	13	16	50	-20		
GR. CLY. SI. S.A.	1.03	10.0	100 32	34	P	16	50	-25		
GR. SI. CL. W/1% OGD- 570 S.A.	1.07	10.0	100 30	35	23	50	-60			
GR. CL. W/1% OGD	1.66	10.0	100 31	33	13	50	-70			
GR. CLY. SI. SILT	0.35	10.0	100 31	33	16	50	-80			



NOTES	WEIGHT SHEETS	PROJECT	DESIGN	NO.
	52	808-10-03	ASSUMPTION	\$52-24
BL.	= Blue	CL.	= Clay	SL.
BK.	= Black	GRAV.	= Gravel	SLA.
BR.	= Brown	ORG.	= Organic	TR.
GR.	= Gray	SA.	= Sand	TP.
GN.	= Green	SH.	= Shelly	PENIT.
PK.	= Pink	SI.	= Silt	H.C.
WH.	= White	ALT.	= Alternating	S.S.
YE.	= Yellow	CO.	= Concrete	VLD.
CONC.	= Concentric	FI.	= Fine	M.S.
ITS.	= Roots	MED.	= Medium	M.V.S.
ROT.	= Rotten	LAM.	= Lamination	40° S.
VEG.	= Vegetation	LEN.	= Lengths	SL.
WD.	= Wood	LY.	= Layers	SLA.
				E 53
MOISTURE CONTENT				
	Moisture Content of the material in its natural state expressed as a percentage of the dry weight of the material			
(3)	<ul style="list-style-type: none"> <li>Location and Identification of the sample</li> </ul>			
NO PENETRATION	<ul style="list-style-type: none"> <li>Unable to drive split spoon sampler initial 6 inches</li> </ul>			
N = 10	<ul style="list-style-type: none"> <li>Number of blows of 140 lbs. hammer dropped 30 in. required to drive 2 in. O.D. Split Spoon sampler 1 in. after first having been driven 6 in. unless amount of penetration shown otherwise</li> </ul>			
NO CULL	<ul style="list-style-type: none"> <li>No preliminary 6" driving prior to securing drive data</li> </ul>			
%	<ul style="list-style-type: none"> <li>Compressive strength from triaxial undrained shear test (tons per sq. ft.)</li> </ul>			
qc	<ul style="list-style-type: none"> <li>Compressive strength from consolidated undrained shear test (tons per sq. ft.)</li> </ul>			
f	<ul style="list-style-type: none"> <li>Angle of internal friction</li> </ul>			
w	<ul style="list-style-type: none"> <li>Wet weight of in-place material (tons per cu. ft.)</li> </ul>			
	<ul style="list-style-type: none"> <li>Strength determined by vane shear test</li> </ul>			
E 4"	<ul style="list-style-type: none"> <li>Estimated unconfined compressive strength by correlation with similar material on this project.</li> </ul>			

CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES				
SOIL	DESIGNATION	N <sup>o</sup> OF BLOWS "N"	UNCONFINED COMPRESSIVE STRENGTH	TONS PER SQ FT
SAND	RELATIVE CONSISTENCY	VERY LOOSE LOOSE MEDIUM DENSE VERY DENSE	LESS THAN 4 4-10 10-30 30-50 OVER 50	
SILT				
CLAY	CONSISTENCY	VERY SOFT SOFT MEDIUM STIFF VERY STIFF HARD	LESS THAN 2 2-4 4-8 8-16 16-30 OVER 30	LESS THAN 0.25 0.25-0.50 0.50-1.00 1 2-4 OVER 4

STATION (ELEV.)	PLAN TIP ELEV.	CUT- OFF ELEV.	PLAN PILE LENGTH	ORDER LENGTH	AS BUILT TIP ELEV.		
					MAX. ELEV.	MIN. ELEV.	AVE. ELEV.
1 11+68	-36.17	18.83	55'	55'			
2 11+87	-46.17	18.83	65'	65'			
3 12+06	-51.17	18.83	70'	70'			
4 12+25	-56.17	18.83	75'	75'			
5 12+44	-56.17	18.83	75'	75'			
6 12+63	-56.17	18.83	75'	75'			
7 12+82	-56.17	18.83	75'	75'			
8 13+01	-51.17	18.83	70'	70'			
9 13+20	-46.17	18.83	65'	65'			
10 13+39	-36.17	18.83	55'	55'			

**CORE BORINGS AND TEST PILES.**

**BAYOU LAFOURCHE BRIDGE  
AT  
PAINCOURTVILLE**

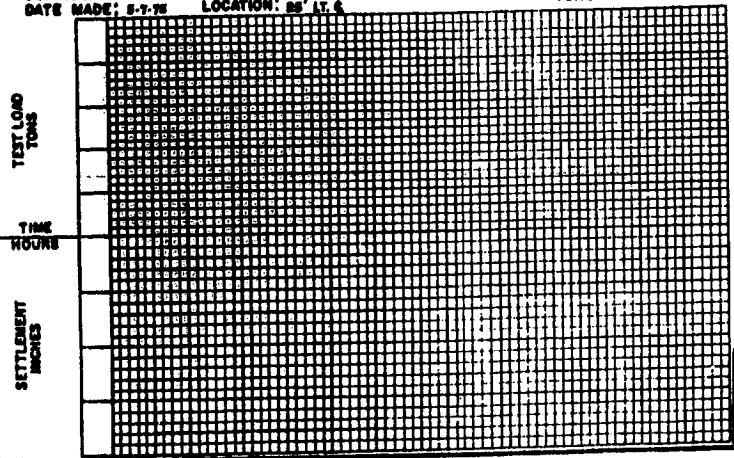
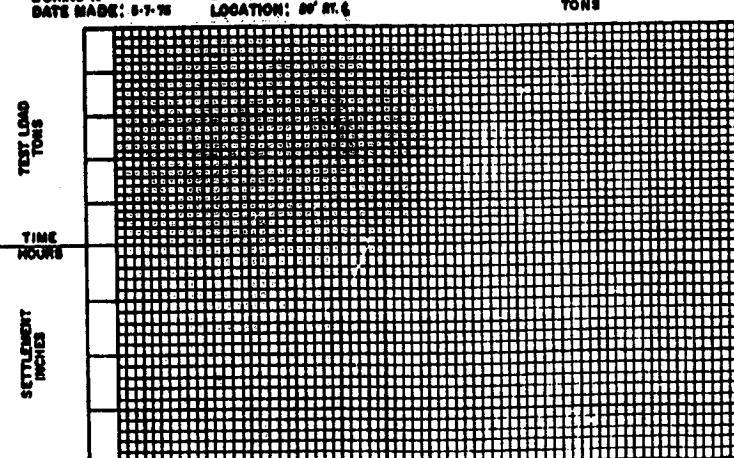
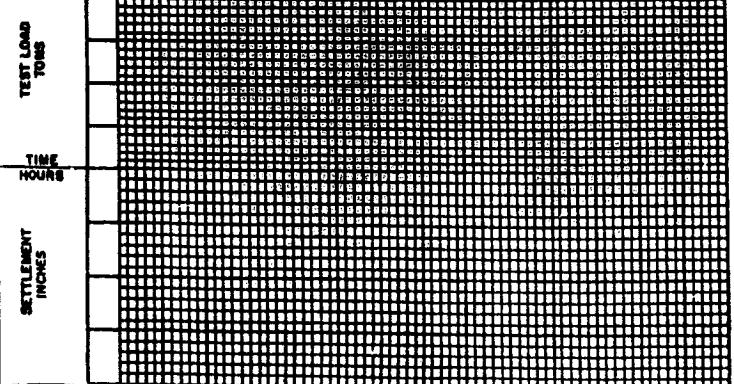
DATED APR. 5. 1971

**STATE OF LOUISIANA  
DEPARTMENT OF HIGHWAYS**

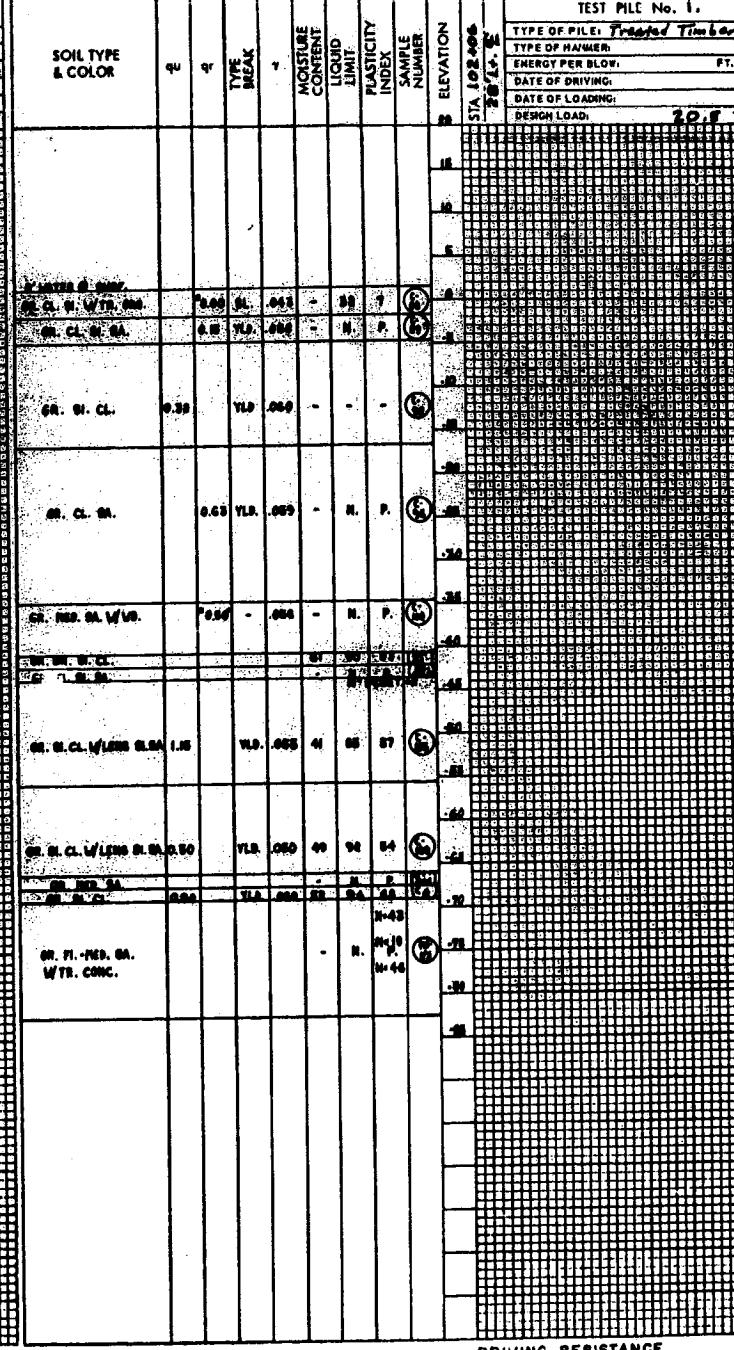
**STATE OF LOUISIANA  
DEPARTMENT OF HIGHWAYS**

**TESTS & TESTING SECTION** — **BRIDGE DESIGN SECTION**

SOIL TYPE & COLOR	QU	QT	TYPE BREAK	Y	MOISTURE CONTENT	LIQUID LIMIT	PLASTICITY INDEX	SAMPLE NUMBER	ELEVATION	TEST PILE No.	
										STA	FT. LBL
(W/TR. WB. & SNG.) @ EL. +13.4 - +13.4											
GR. DR. CL. SI.			0.78	N.S.	.065	-	N.	P.	(5)		
GR. CL. SI.			0.86	N.S.	.061	-	N.	P.	(5)		
GR. SI. CL. W/STR. SI.	1.18			R.S.	.069	34	37	14	(5)		
GR. DR. PI. SI. SA.											
GR. CL. W/ GR. LENS											
GR. SI. CL.	1.27		YLD.	.066	29	40	19	(5)			
	1.48				87						



SOIL TYPE & COLOR	QU	QT	TYPE	BREAK	MOISTURE CONTENT	LIQUID LIMIT	PLASTICITY INDEX	SAMPLE NUMBER	TEST PILE No.	
									TYPE OF FILE	TYPE OF HAMMER
GR. S. CL. 60. 65.	1.00		N.C.	NO	-	31	11	(1)		FT. L.
GR. CL. SI. SA.			CL	NO	-	31	11	(2)		
GR. CL. SI. SA.			CL	NO	-	31	11	(3)		
GR. CL. SI.	0.90		P.E. 500	-	30	9	(4)			
GR. S. CL. 60. 65.	1.00		N.C.	NO	-	33	9	(5)		
GR. S. CL. 60. 65.	1.00		N.C.	NO	-	31	11	(6)		
GR. S. CL.					-	31	11	(7)		
GR. S. CL.					-	31	11	(8)		
GR. S. CL.	0.95		V.G. 500	NO	33	11	12	(9)		
GR. S. CL.	0.70		V.L. 500	NO	30	10	14	(10)		
GR. S. CL. SI. CL.	0.91		V.L. 500	NO	34	10	16	(11)		
GR. CL. SI.					-	31	11	(12)		



NOTES	PROJECT	PANISH	SKETCH
	804-11-06	Assumption	112
BL.	= Blue	CL.	= Clay
BK.	= Black	GRAV.	= Gravel
BR.	= Brown	ORG.	= Organic
GR.	= Gray	SA.	= Sand
GN.	= Green	SH.	= Shelly
PK.	= Pink	SI.	= Silty
WH.	= White	ALT.	= Alternating
YE.	= Yellow	CO.	= Coarse
CONC.	= Concrete	FI.	= Fine
RTS.	= Roots	MED.	= Medium
ROT.	= Rotten	I. O.	= Iron Ore
VEG.	= Vegetation	LEM.	= Lenses
WD.	= Wood	Q. A.	= Quick Acting
MOISTURE CONTENT	= Moisture Content of the material in its natural state expressed as a percentage of the dry weight of the material		
(3)	= Location and identification of the sample		
NO PENETRATION	= Unable to drive split spoon sampler initial 6 inches		
N = 10	= Number of blows of 140 lbs. hammer dropped 30 in. required to drive 2 in. O.D. Split Spoon sampler 1 ft. after first having been driven 6 in. unless amount of penetration shown otherwise		
NO CULL	= No preliminary 6" driving prior to securing drive data		
¶	= Compressive strength from triaxial undrained shear test (tons per sq. ft.)		
¶	= Compressive strength from consolidated undrained shear test (tons per sq. ft.)		
¶	= Angle of interfacial friction		
¶	= Wet weight of in-place material (Tons per cu. ft.)		
¶	= Strength determined by PENETROMETER		
¶ 4	= Estimated unconfined compressive strength by correlation with similar material on this project.		

CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES				
SOIL	DESIGNATION	N <sup>o</sup> OF BLOWS "N"	"q <sub>u</sub> " UNCONFINED COMPRESSIVE STRENGTH TONS PER SQ.FT.	
SAND & SILT	PERMEATIVE DENSITY	VERY LOOSE LOOSE MEDIUM DENSE VERY DENSE	LESS THAN 4 4-10 10-30 30-50 OVER 50	
CLAY	CONCRETE TEST	VERY SOFT SOFT MEDIUM STIFF VERY STIFF HARD	LESS THAN 2 2-4 4-8 8-15 15-30 OVER 30	LESS THAN 0.25 0.25 - 0.50 0.50 - 1.00 1 - 2 2 - 4 OVER 4

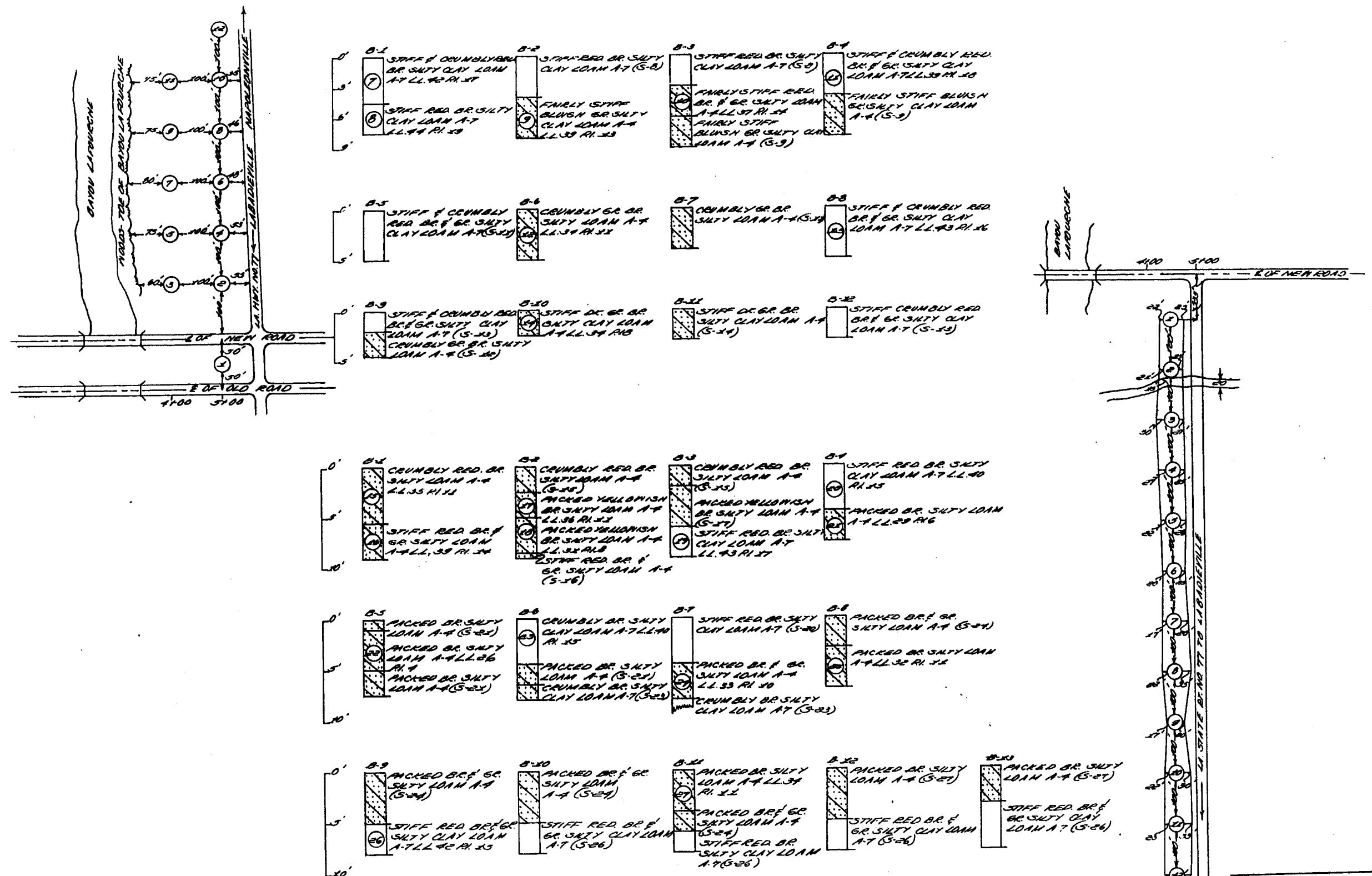
### CORE BORINGS AND TEST PILES.

**BAYOU LAFOURCHE BRIDGE  
AT MUNSON  
LA. 402**

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STATE OF LOUISIANA  
DEPARTMENT OF HIGHWAYS

7



BORROW PIT MAPS

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STATE

SIA DEPARTMENT

**DEPARTMENTS**

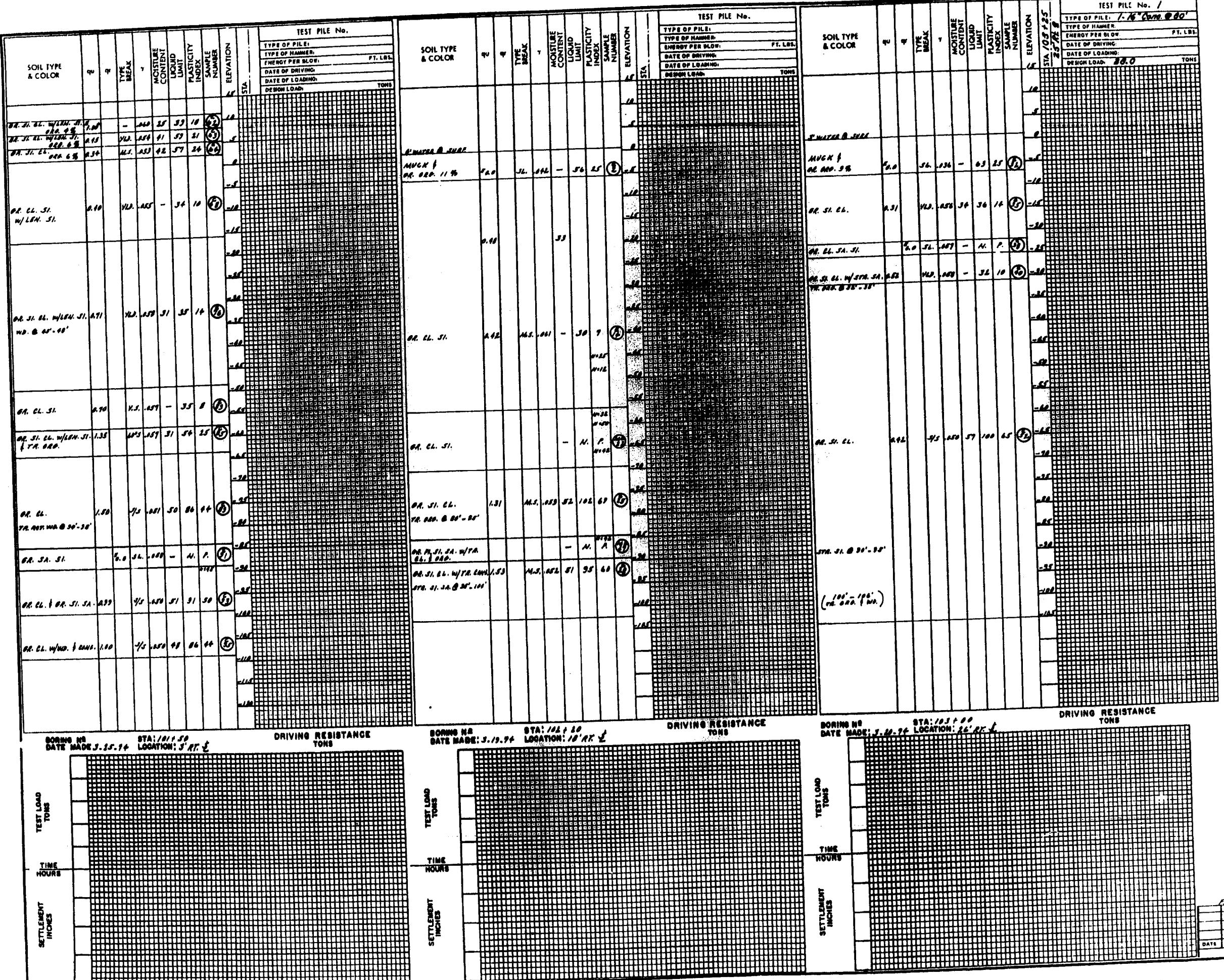
SEARCHED **J.M.**

CHARGE OF H.L. LEHM

*410-10-33*

**STATE OF LOUISIANA  
DEPARTMENT OF HIGHWAYS**

NONED UNF.	DETAILED <i>SPK.</i>	TRACED <i>CON.</i>
SEARCHED <i>JAN.</i>	CHECKED <i>MED.</i>	CHECKED <i>MED.</i>
CHARGE OF M.L. LEHMANN-TESTING & RESEARCH EN		



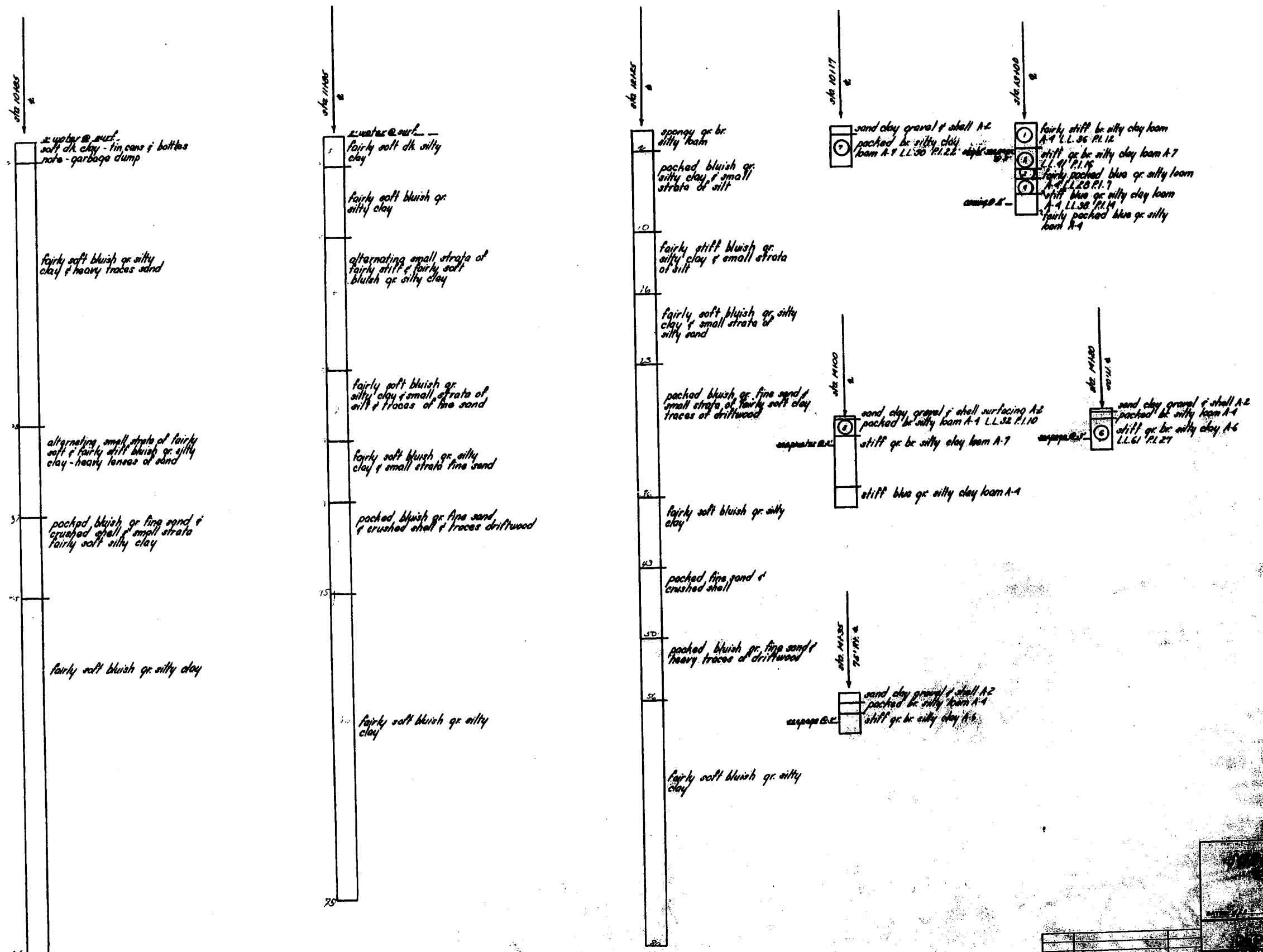
		A-1 1973-02-01		B-1 1973-02-01		SHEET 12	
		739-02-01 ASSUMPTION					
Blue	C.	Clay	Silt	Sands	Mud		
Black	CLAY	Clayey	Silt	Loamy	Fine		
Brown	CLAY	Clayey	Silt	Loam	Fine-Loam		
Gray	SA.	Sand	Cl. S.	Peat	Peat		
Green	SI.	Shelly	CL. S.	Clay	Clay		
Pink	SI.	Silty	CL. S.	Clay	Clay		
White	AL.	Alternating	S.S.	Slackmud			
Yellow	CO.	Clayey	Y.L.	Yield			
Concretes	FI.	Fine	N. S.	M. Triple Shear			
Roots	M.D.	Medium	S. S.	Zone of Shear			
Rotten	I. O.	Iron Ore	G. S.	Shear Angle			
Vegetation	LEN.	Lenses	S.	Slip			
Wood	O. A.	Quick Acting					
<b>22</b>							
<b>E CONTENT</b>							
Moisture Content of the material in its natural state - expressed as a percentage of the dry weight of the material							
<b>EXPLORATION</b>							
Location and Identification of the sample							
Unable to drive split spoon sampler initial 6 inches							
Number of blows of 100 lbs. hammer dropped 30 in. required to drive 2 in. O.D. Split Spoon sampler 1 ft., after first having been driven 6 in., unless amount of penetration shown otherwise							
No preliminary 6" driving prior to recording drive data							
Compressive strength from triaxial unstrained shear test (tons per sq. ft.)							
Compressive strength from consolidated undrained shear test (tons per sq. ft.)							
Angle of internal friction							
Wet weight of in-place material (tons per cu. ft.)							
Strength determined by <b>PENETROMETER</b>							
Estimated unconfined compressive strength by correlation with similar material on this project.							
<b>CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES</b>							
<b>SOIL</b>		<b>DESIGNATION</b>		<b>N<sup>o</sup> OF BLOWS "N"</b>		<b>"E<sub>u"</sub> UNCONFINED COMPRESSIVE STRENGTH TONS PER SOFT FT.</b>	
<b>AND SILT</b>	<b>RELATIVE DENSITY</b>	VERY LOOSE		LESS THAN 4			
		LOOSE		4-10			
<b>CLAY</b>	<b>CONSISTENCY</b>	MEDIUM		10-30			
		DENSE		30-50			
		VERY DENSE		OVER 50			
		VERY SOFT		LESS THAN 2		LESS THAN 0.25	
		SOFT		2-4		0.25-0.50	
		MEDIUM		4-8		0.50-1.00	
		STIFF		8-15		1-2	
		VERY STIFF		15-30		2-4	
		HARD		OVER 30		OVER 4	

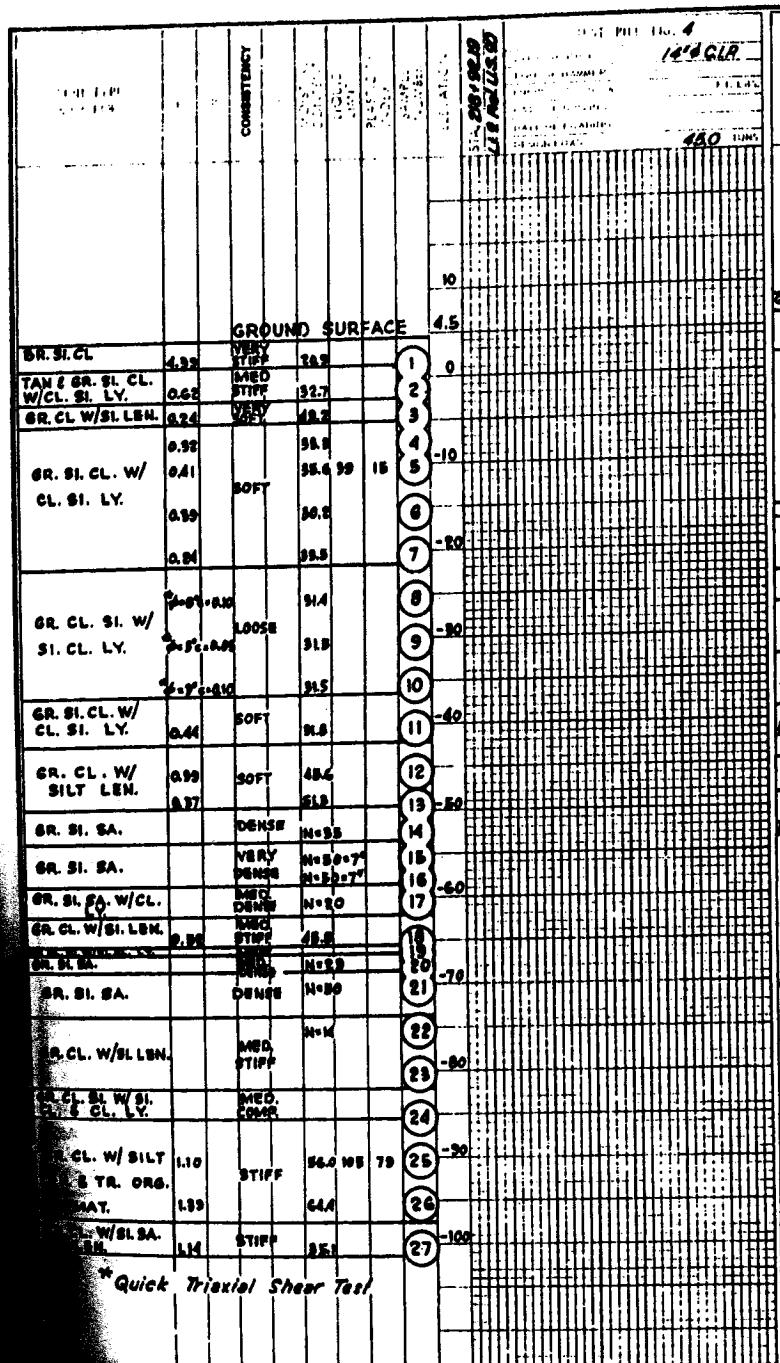
PILE DATA							
CATION IDENT	STATION	PLAN TIP ELEV.	CUT- OFF ELEV.	PLAN FILE LENGTH	ORDER LENGTH	AS BUILT TIP ELEV.	
						MAX. ELEV.	MIN ELEV.
1	101+57	-33.98	16.02	50'			
2	101+76	-38.98	16.02	55'			
3	101+95	-48.98	16.02	65'			
4	102+14	-53.98	16.02	70'			
5	102+33	-58.98	16.02	75'			
6	102+52	-58.98	16.02	75'			
7	102+71	-58.98	16.02	75'			
8	102+90	-58.98	16.02	75'			
9	103+09	-58.98	16.02	75'			
10	103+28	-48.98	16.02	65'			
11	103+47	-38.98	16.02	55'			
12	103+66	-33.98	16.02	50'			

CORE BORINGS AND TEST PILES		
BAYOU LAFOURCHE BRIDGE (SUPREME)		
LA.1011		
DATED APRIL 17 1974		
STATE OF LOUISIANA DEPARTMENT OF HIGHWAYS		
SURVEYED <i>M. FASTER</i>	DETAILED <i>CHEMEN</i>	TRACED <i>LEWIS</i>
CHECKED <i>M. FASTER</i>		CHECKED <i>LEWIS</i>
MATERIAL SECTION		BRIDGE DESIGN

10

BAYOU LAFOURACHE BRIDGE -  
LABADIEVILLE  
STATE PROJ. 804-35-51  
ASSUMPTION PARISH





W C-121 STA: 219+00  
MADE 7/18/67, 9/69 LOCATION: 50'

ME C-121 STA: 219+00  
MADE 7/28/67 BY LOCATION: 50' LT.

**DRIVING RESISTANCE  
TONS**

BORING NO C-121 Cont. STA:  
DATE MADE: LOCATI

**DRIVING RESISTANCE  
TONS**

BORING NO. STA.  
DATE MADE: LOCATION:

**DRIVING RESISTANCE  
TONS**

The figure consists of two side-by-side graphs. Both graphs have 'TEST LOAD TONS' on the vertical axis and 'TIME HOURS' on the horizontal axis. The left graph has 'SETTLEMENT INCHES' on its vertical axis. The right graph has 'SETTLEMENT INCHES' on its vertical axis. In both graphs, the settlement increases with time and load. The right graph shows a higher rate of settlement than the left graph.

CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES				
SOIL	DESIGNATION	N <sup>o</sup> OF BLOWS 'N'	"E <sub>u</sub> " UNCOMPRESSIVE STRENGTH LBS PER SQFT	"E <sub>u</sub> " UNCOMPRESSIVE STRENGTH LBS PER SQFT
SAND OR SILT	VERY LOOSE	LESS THAN 4		
	LOOSE	4-10		
	MEEDIUM	10-30		
	DENSE	30-50		
	VERY DENSE	OVER 50		
CLAY	VERY SOFT	LESS THAN 2	LESS THAN 0.25	0.25-0.50
	SOFT	2-4	0.50-1.00	0.50-1.00
	MEDIUM	4-9		
	STIFF	9-15		
	VERY STIFF	15-30		
	HARD	OVER 30		OVER 4

**NOTE: The Test Pit Cut-Off Elevation is 8.0' (Approx.) Above  
The Existing Ground.**

BORE BORINGS AND TEST PILES

**RELOCATION OF U.S. 90  
(RACELAND - GIBSON)**

**EUSTIS ENGINEERING COMPANY  
SOIL AND FOUNDATION CONSULTANTS**

**STATE OF LOUISIANA  
DEPARTMENT OF HIGHWAYS**

DEPARTMENT OF HIGHWAYS

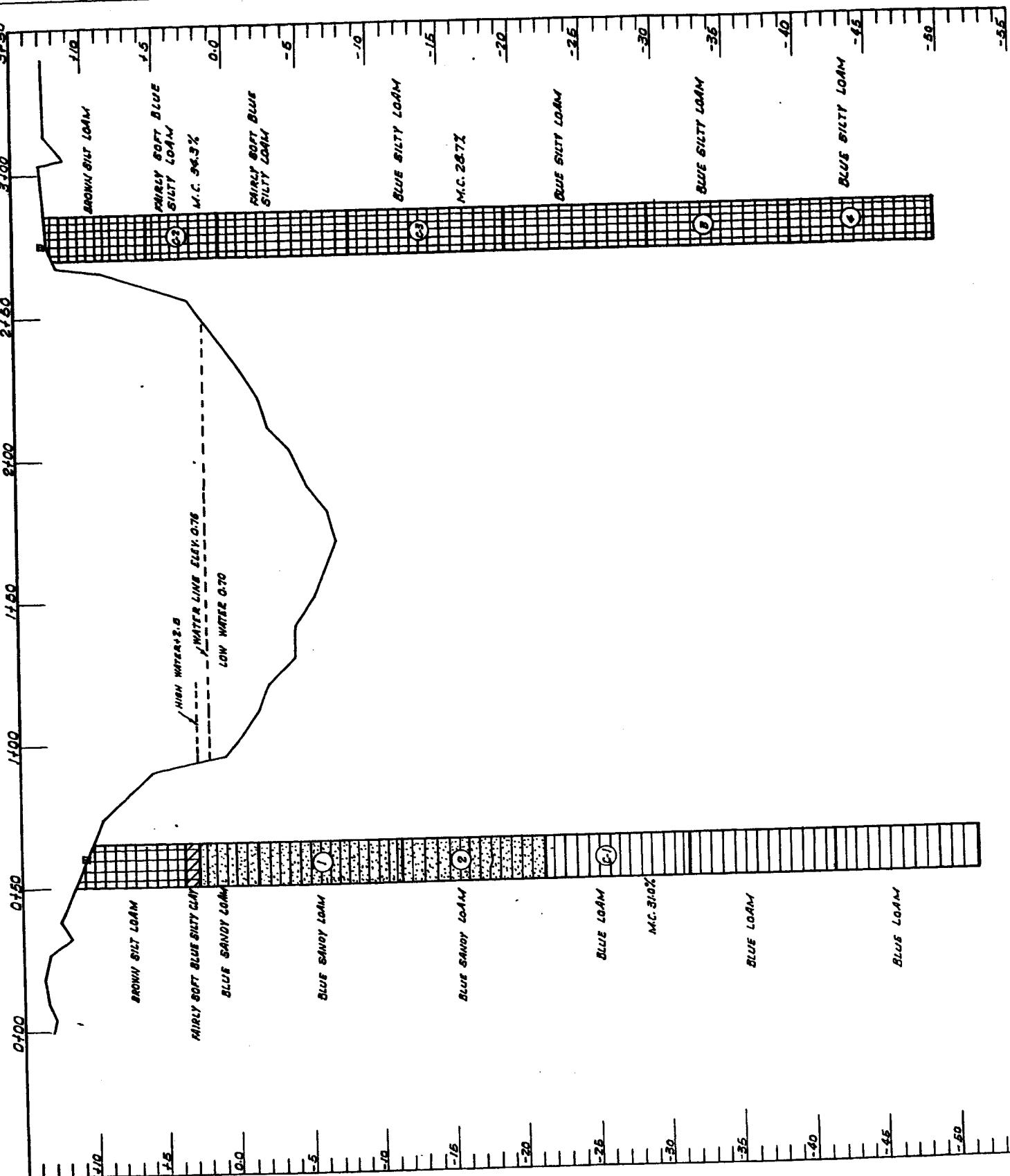
**SEARCHED** **SEARCHED**

**TESTING SECTION — BRIDGE DESIGN SECTION**

FIGURE 7

**SOIL SURVEY  
STATE PROJ. 931-00-23  
BAYOU LAFOURCHE PONTOON BRIDGE  
LAFOURCHE PARISH**

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## **FINAL TRACINGS**



**Note**

- ... CENTER OF DRILLING
- M.G. 23.6% ... Motions Content of Sample
- (1) ... CORE SAMPLE NUMBER & LOCATION
- (2) ... SAMPLE # & Location (disturbed cores)

